

# McLeod Development Inc.

---

## Functional Servicing Report

---

**Project Location :**

6645, 6655, 6665 McLeod Road

Niagara Falls, Ontario

**Prepared for:**

-

**Prepared by:**

AM Engineering

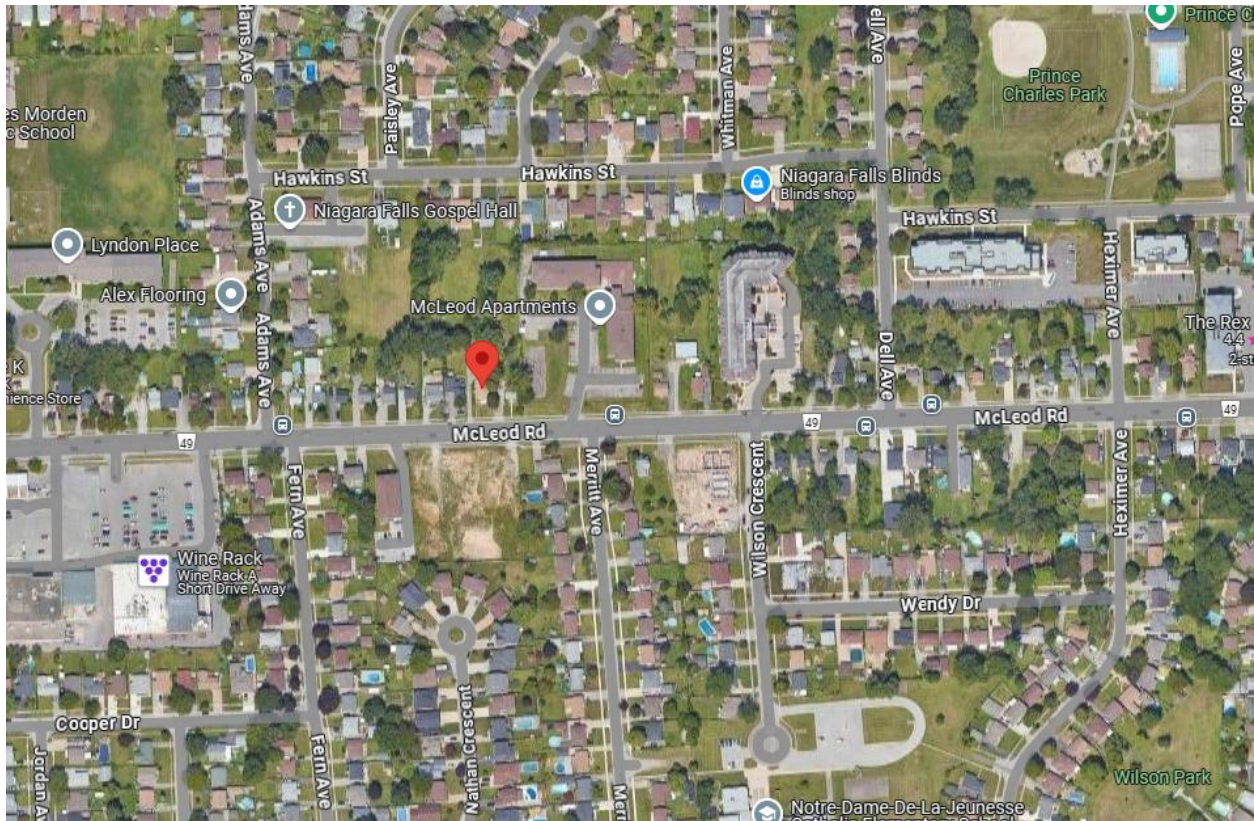
Jan, 2025

**File No: 24083**

<b>TABLE OF CONTENTS</b>	<b>Page</b>
1.0 INTRODUCTION.....	2
2.0 SANITARY SERVICING.....	3
2.1 Existing Sanitary Servicing.....	3
2.2 Proposed Sanitary Servicing and Peak Discharge Rates.....	3
2.2.1 Apartment Occupancy (Table 8.2.1.3.A).....	3
2.3 Sewer Full Flow Capacity:.....	4
3.0 WATER.....	5
3.1 Existing Conditions.....	5
3.2 Water Usage.....	5
3.3 Fire Flow Demand Requirements.....	7
3.4 Available Fire Flow.....	8
4.0 STORMWATER SERVICING.....	9
5.0 SUMMARY.....	9

## 1.0 INTRODUCTION

This report addresses the existing and proposed Site Servicing in support of a proposed building and associated parking/driveways on the subject site. The proposed structure is a Stacked Townhouse development comprising three blocks. Block A has 18 units, Block B, 12 Units and Block C, 12 units. The subject site is located at 6645, 6655 & 6665 McLeod Road, Niagara Falls. The site is located north of McLeod Road. **(See Figure 1).**



**Figure 1: Location Map**

## 2.0 SANITARY SERVICING

### 2.1 Existing Sanitary Servicing

The property is currently occupied by a single family residential and is serviced with municipal water and sanitary. The site has an area of 4220m<sup>2</sup> (0.422 ha).

City records indicate that existing sanitary on McLeod include 525mm and 600 diameter sewers. Refer to conceptual layout sketch in the Appendix.

All redundant services to the property shall be capped at the property line by the Developer's Contractor.

### 2.2 Proposed Sanitary Servicing and Peak Discharge Rates

It is proposed to construct three blocks of Stacked Townhouses as well as parking and driveway to service the development.

The proposed peak sanitary discharge generated from the development has been calculated as follows:

The proposed peak sanitary discharge generated from the development has been calculated based on Table 8.2.1.3.A of the 2024 Ontario Building Code (OBC)

#### 2.2.1 Apartment Occupancy (Table 8.2.1.3.A)

- Apartment - @ 275 L/day per person
  - Population (apartments) = 1.6 persons per Unit x 42 Units = 68 pple
- For 42 units (18+12+12) = [42 x 1.6 x 275]=18480 L/day = 0.04 L/s

Total Average Flow = 0.214 L/s

Peaking Factor =  $1 + \frac{14}{4+P^{0.5}} = 1 + \frac{14}{4+0.068^{0.5}} = 4.28$

Therefore use 4.0 (max allowed)

Design Peak Discharge = 0.214 x 4.00 = **0.856 L/s**

Infiltration Amount = Catchment Area x 0.286 L/s/ha  
= 0.286 L/s/ha x 0.4220 ha = **0.121 L/s**

Peak Discharge = Residential Flow + Infiltration Amount  
= 0.856 L/s + 0.121 L/s  
= **0.977 L/sec**

### 2.3 Sewer Full Flow Capacity:

It is proposed to provide a 150mm diameter sewer at a minimum 0.5% slope off the 525mm diameter sewer on McLeod Road. The calculation for sewer full flow capacity is as follows:

$$Q = \frac{1}{n} \times A \times R^{\frac{2}{3}} \times S^{\frac{1}{2}}$$

Where:

Q = Design Flow (m<sup>3</sup>/s)

'n = Mannings roughness coefficient

A = Cross sectional area of flow (m<sup>2</sup>)

R = Hydraulic radius (Area/wetted perimeter)

S = Slope of pipe (m/m)

The existing sewer is a 150mm diameter PVC at 0.5% slope.

$$Q = \frac{1}{0.013} \times 0.0177 \times 0.0375^{\frac{2}{3}} \times 0.005^{\frac{1}{2}} = 0.01077m^3/s = 10.77l/s$$

The sanitary sewer has adequate capacity for the expected peak flow (10.77 L/s > 0.977 L/s).

## **3.0 WATER**

### **3.1 Existing Conditions**

City records indicate that a 200mm diameter watermain is available on McLeod Road as well as 25mm copper K water services to the property. These services shall be capped at the main by City's Public Works Department at the Developer's expense.

### **3.2 Water Usage**

Peak water usage for the proposed development is estimated based on the "Fixture Unit Method" as per Table 7.6.3.2.A of the Ontario Building Code (OBC) 2024. The fixture units were estimated based architectural floor plans.

**Table 1: Fixture Units Calculation:**

Fixture or Device	Minimum Size of Supply Pipe	Private Use Hydraulic Load (fixture units)	Quantity	Total Hydraulic Load (fixture units)
Bathroom group <sup>(a)</sup> with 6 LPF flush	N/A	3.6	58	208.8
Bathtub with or without shower head	1/2"	1.4		
Bathtub with ¾ in. spout	3/4"	10		
Bidet	3/8"	2		
Clothes washer	1/2"	1.4	42	58.8
Dishwasher	3/8"	1.4	42	58.8
Water closet, 6 LPF or less with flush	3/8"	2.2		
<b>Hose bibb:</b>				
½" supply	1/2"	2.5		
¾" supply	3/4"			
Combination hot and cold	1/2"	2.5		
<b>Shower head:</b>				
Single head	1/2"	1.4		
Multi-head, fixture unit per head	1/2"	1.4		
<b>Sink:</b>				
Bathroom (lavatory)	3/8"	0.7	10	7
Bar	3/8"	1		
Kitchen	3/8"	1.4	42	58.8
Laundry	3/8"	1.4	42	58.8
Other:				
<b>Total:</b>				<b>451.0</b>

Conversion of fixture units to flow (L/s) was done as per Table 7.4.10.5

**451.0 Fixtures Units = 108.63 gal/min = 6.85 L/s**

### 3.3 Fire Flow Demand Requirements

Fire flow requirements for the proposed development have been estimated using the Fire Underwriter's Survey: *Water Supply for Public Fire Protection - 1999*. The calculations are provided below:

#### 1. Building Construction

Proposed buildings are un-sprinkled and combustible construction.

#### 2. Base Fire Flow

$$F = \text{base fire flow requirement (L/min)} \\ = 220 \times C \times A^{0.5}$$

$$A = 910 \text{ sq.m (floor area) [2 adjoining floor + 50\% floor above - > 1 hour rating]}$$

$$C = \text{fire resistance coefficient} \\ = 1.0 \text{ (combustible construction)}$$

$$F = 220 \times 1.0 \times 910^{0.5} = 6636.6 \text{ l/min} = 7000 \text{ L/min [to the nearest 1000 L/min]}$$

#### 3. Adjustment for Sprinkler System

Proposed building shall not be equipped with a sprinkler system.

No reduction due to sprinkler system

#### 4. Adjustment for Adjacent Buildings

$$\text{East face within 10.1-20m of building} = \text{add 15\% of Adjusted F}$$

$$\text{West face within 10.1-20m of building} = \text{add 15\% of Adjusted F}$$

$$\text{South face >45m of building} = \text{add 0\% of Adjusted F}$$

$$\text{North face >45m of building} = \text{add 0\% of Adjusted F}$$

$$\text{Total} = 30\% \text{ of Adjusted F (Max Value)}$$

$$= 30\% \times 7000 \text{ l/min} = 2100 \text{ l/min}$$

$$5. \text{ Final Fire Flow Calculation} = 7000 + 2100$$

$$= 9,100 \text{ L/min}$$

$$= 9,000 \text{ L/min [rounded the nearest 1000 L/min]}$$

$$= \mathbf{150 \text{ L/s}}$$



### 3.4 Available Fire Flow

A fire hydrant is located on McLeod Road directly opposite the proposed entrance to the property. Fire flow test results (attached) indicate that the required flow of 150 L/s would be available at an estimated 40 psi.

#### Figure 2: Fire Hydrant

The unobstructed distance between the existing fire hydrant on McLeod Road and the main entrances to the Block C building is approximately 112m. This exceeds the requirements of the Ontario Building Code section 3.2.5.5 that stipulates this distance should not exceed 90m. It therefore proposed to provide a private Fire Hydrant as shown on the conceptual layout sketch to satisfy the OBS requirement.



## 4.0 STORMWATER SERVICING

Storm drainage for the proposed conditions shall be provided through surface and underground drainage systems. The minor system comprises swales and storm sewers. Major flows shall be directed to McLeod Road. Stormwater Management is discussed in detail in a SWM report provided under a separate cover.

## 5.0 SUMMARY

The main findings of the functional servicing report for the proposed development are:

1. The property can be adequately serviced through the proposed sanitary and water services.

**AM Engineering.**



Gurbir Mundi, P.Eng.

## **APPENDIX A**

### **FIRE HYDRANT TEST**

### Water Supply Test

Name of Risk: Mcloed Development Inc File No: 240633  
Address: 6655 McLeod Road Tested By: D. Boyes  
Municipality: Niagara Falls Date: December 16<sup>th</sup>, 2024

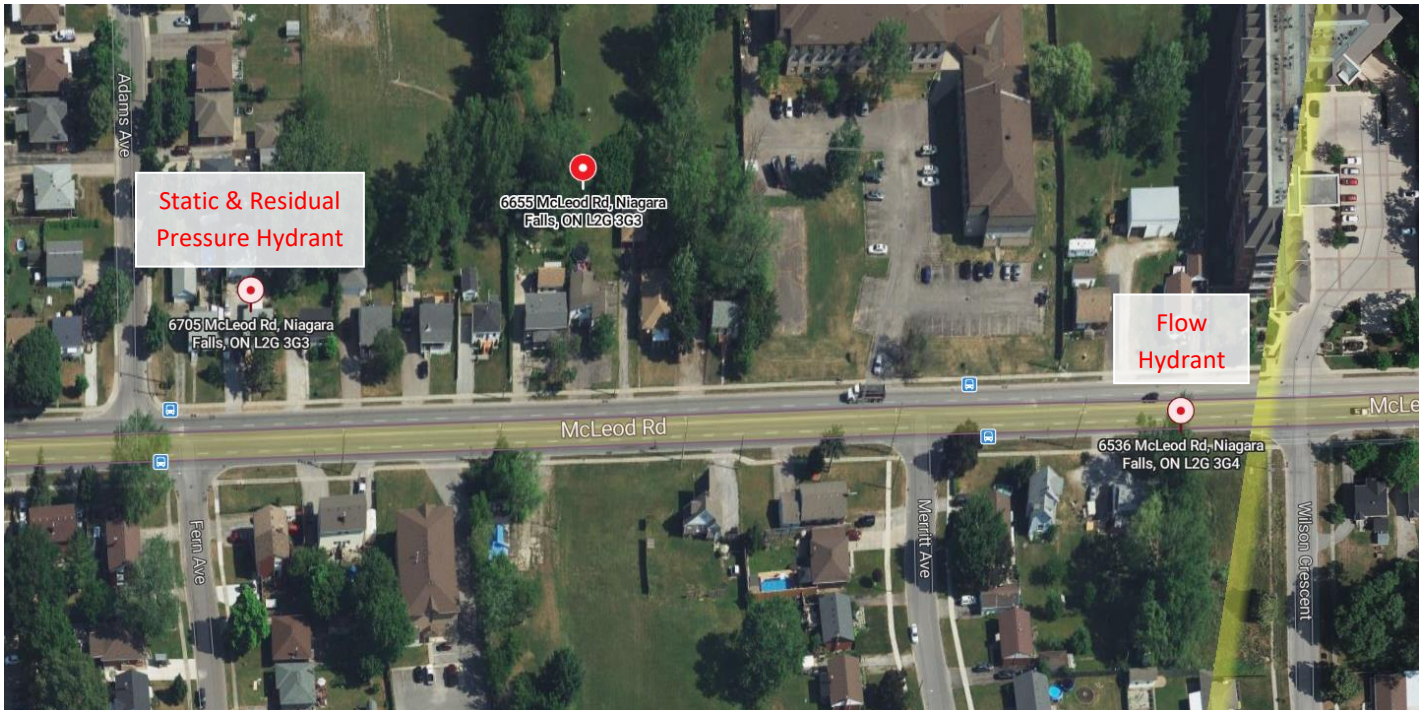
### System Data

Size of Main: 300mm PVC Dead End: \_\_\_\_\_ Two Ways:   x   Loops: \_\_\_\_\_  
Source Reliable: Yes City If No Explain: \_\_\_\_\_  
Comments: Hydrant located @ 6655 McLeod was out of service at time of flow test.

### Test Data

Location of Test Hydrants: Static: 6705 McLeod Road  
Flowed: 6536 McLeod Road  
Static: 92 psi Time: 8 A.M

Test No:	No. of Outlets	Orifice Size	Pitot PSI	Equivalent Flow GPM	Total Flow GPM	Residual Pressure Psi	Comments
1	1	2 ½"	52	1216	1216	90	Hose Monster
2	2	2 ½"	40/40	1067/1067	2134	87	Hose Monster



## **APPENDIX B**

### **CONCEPTUAL LAYOUT SKETCH**

# CONCEPTUAL WATER AND SANITARY LAYOUT N.T.S

